# B.Tech IV Year II Semester Examinations, April/May-2012 DESIGN AND DRAWING OF HYDRAULIC STRUCTURES (CIVIL ENGINEERING) 

Time: 3 hours
Max. Marks: 70

Answer any ONE question<br>All questions carry equal marks Assume suitable data wherever required. Use of Garrets diagrams, Khosla's Charts, Lacey's Charts are permitted.

1. Design a surplus weir of a work of tank forming a chain of tanks. The combined catchment area of the group of tank is 30 square kilometers and the area of the catchment intercepted by the upper tank is 28 square kilometers.
Top level of the tank bund : +20.00m
Top width of bund : 2.00 m
Side slopes of the tank on either side : $2 \mathrm{H}: 1 \mathrm{~V}$
F.T.L : +20.00

Level of submersion of lands : +20.75
Ground level : +19.00
Assume any other suitable data.
Draw the Plan, L.S and L.E of the weir.
(OR)
2. Design a regulator-cum-road bridge with the following data:

Hydraulic particulars of canal upstream:

| F.S. discharge | $: 22 \mathrm{~m}^{3} / \mathrm{sec}$ |
| :--- | :--- |
| Bed width | $: 15 \mathrm{~m}$ |
| Bed level | $:+32.00$ |
| F.S. Depth | $: 2 \mathrm{~m}$ |
| F.S.L | $:+34.00$ |
| Top level of the bank | $:+35.00$ |

The right bank is 5 m wide and left bank is 2 m wide.

Hydraulic particulars of canal downstream:

| F.S. discharge | $: 15 \mathrm{~m}^{3} / \mathrm{sec}$ |
| :--- | :--- |
| Bed width | $: 15 \mathrm{~m}$ |
| Bed level | $:+32.00$ |
| F.S. Depth | $: 1.80 \mathrm{~m}$ |
| F.S.L | $:+33.75$ |
| Top level of the bank | $:+34.75$ |

Top width of banks are same as those on the $\mathrm{u} / \mathrm{s}$ side. The regulator carries a road way single line designed for IRC loadings class 'A'. Provide clear free board of one meter above F.S.L for the road bridge.

Good foundation is available at +31.00
Assume ground level at site as +34.00 .

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1. Design a surplus weir of a work of tank forming a chain of tanks. The combined catchment area of the group of tank is 40 square kilometers and the area of the catchment intercepted by the upper tank is 30 square kilometers.

Top level of the tank bund $\quad:+22.00 \mathrm{~m}$
Top width of bund $: 2.00 \mathrm{~m}$
Side slopes of the tank on either side : $2 \mathrm{H}: 1 \mathrm{~V}$
F.T.L : +20.00

Level of submersion of lands : +20.75
Ground level : +19.00
Assume any other suitable data.
Draw the Plan, L.S and L.E of the weir.
(OR)
2. Design a sluice (tank sluice with tower head) taking off from a tank with a following data: of

Discharge
Top width of the bund
$: 0.35 \mathrm{~m}^{3} / \mathrm{sec}$

Side slopes
: 2.0 m
Top level of the bank
: 2: 1
Ground level at the site
: +51.00
$:+45.50$
Sill of the sluice at off-take is $:+45.00$
Maximum water level in the tank $:+49.00$
Full tank level is $:+48.00$
Average low water level is $:+46.00$

Good hard soil for foundation is available at +44.50 .
Details of canal below the sluice

Bed level
F.S.L
: +45.50
Bed width
: 1.50 m
Side slopes
: $11 / 2$ to 1 with top of bank at +46.50

Draw the longitudinal section. Assume any other suitable data.

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1. Design a sluice (tank sluice with tower head) taking off from a tank with a following data:
Discharge
: $0.20 \mathrm{~m}^{3} / \mathrm{sec}$
Top width of the bund
: 2.0 m
Side slopes
: 2: 1
Top level of the bank
: +48.00
Ground level at the site
: +42.50
Sill of the sluice at off-take is
: +42.00
Maximum water level in the tank
: +46.00
Full tank level is
$:+45.00$
Average low water level is
$:+43.00$

Good hard soil for foundation is available at +41.50 .
Details of canal below the sluice
Bed level
$:+42.00$
F.S.L
: +42.50
Bed width
: 1.30 m
Side slopes $: 1 \frac{1}{2}$ to 1 with top of bank at +43.50
Draw the longitudinal section. Assume any other suitable data.

## (OR)

2. Design a siphon aqueduct (type-III) for forming the canal through an RCC trough with the following data:

Discharge of the canal : 32 cumecs
Bed width of the canal : 20 m
Depth of water in the canal $: 1.5 \mathrm{~m}$
Bed level of the canal $:+160.50 \mathrm{~m}$
High flood discharge of the drainage : 400 cumecs
High flood level of the drainage $\quad:+161.00 \mathrm{~m}$
Bed level of the drainage $\quad:+158.50 \mathrm{~m}$
General ground level $\quad:+160.50 \mathrm{~m}$
Canal free board $: 1.0 \mathrm{~m}$
Canal side slopes both inside and outside are 2: 1 in embankment
Top width of the bank on left side is 5 m carries a road way while the top width on the right bank is 3 m .

The foundations of abutments and respective wing-wall and returns both on the $\mathrm{u} / \mathrm{s}$ and $\mathrm{d} / \mathrm{s}$ side are taken to the respective scour depth levels assuming silt factor as 1.0. Hard soil fit for foundation is available below at +156.50 m .

Limiting velocity in the drainage not to exceed $2.5 \mathrm{~m} / \mathrm{sec}$.
The velocity in the canal trough is not to exceed twice the normal velocity in the canal. Find out the loss of head in the canal due to the crossing by total energy line method. Similarly find out the H.F.L of the drain $\mathrm{u} / \mathrm{s}$ of the structure, keeping the normal H.F.L of the drain at the $\mathrm{d} / \mathrm{s}$ side returns of the drainage barrel.

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Khosla's Charts, Lacey's Charts are permitted.

1. Design a canal drop of 2 m of trapezoidal notch type with the following data:

Canal U/s $\underline{\mathrm{D} / \mathrm{s}}$
Full supply discharge
$5 \mathrm{~m}^{3} / \mathrm{s}$
$+112.0 \mathrm{~m}$
7.0 m
1.50 m
+113.50 m
2.0 m at 114.50
1.0 m .

Half supply depth is
Ground level at the site of the work is +112.50 m .
Good soil is available for foundation is +110.50 m .
Assume any other suitable data. Draw the L.S of the canal drop.

## (OR)

2. Design a concrete sloping glacis weir for the following conditions:

| Maximum discharge intensity on weir crest | $: 10$ cumecs $/ \mathrm{m}$ length |
| :--- | :--- |
| H.F.L before construction of weir | $: 252.00 \mathrm{~m}$ |
| Reduced level of river bed | $: 246.50 \mathrm{~m}$ |
| Pond level | $: 251.00 \mathrm{~m}$ |
| Height of crest shutters | $: 1 \mathrm{~m}$ |
| Bed retrogression | $: 0.5 \mathrm{~m}$ |
| Lacey`s silt factor | $: 0.9$ |
| Permissible exit gradient | $: 1 / 7$ |
| Permissible afflux | $: 1 \mathrm{~m}$ |

Anticipated downstream water level in the river when the weir is discharging with pond level upstream $=249.5 \mathrm{~m}$.

